

Review

Prospects for Biological Nitrogen Removal from Anaerobic Effluents during Mainstream Wastewater Treatment

Jeseth Delgado Vela, Lauren B. Stadler, Kelly J. Martin, Lutgarde Raskin, Charles Bott, and Nancy G. Love Environ. Sci. Technol. Lett., Just Accepted Manuscript • DOI: 10.1021/acs.estlett.5b00191 • Publication Date (Web): 24 Aug 2015 Downloaded from http://pubs.acs.org on August 29, 2015

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1 Prospects for Biological Nitrogen Removal from Anaerobic Effluents during Mainstream

2 Wastewater Treatment

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10 Abstract

11 Growing interest in anaerobic treatment of domestic wastewater requires a parallel focus on 12 developing downstream technologies that address nitrogen pollution, especially for treatment 13 systems located in eutrophication-impacted watersheds. Anaerobic effluents contain sulfide and 14 hydrogen sulfide (a corrosive gas), dissolved methane (a potent greenhouse gas), ammonium and 15 residual organic carbon predominantly in the form of volatile fatty acids. Conventional 16 approaches to nitrogen removal are energy- and chemical-intensive, and are not appropriate for 17 nitrogen removal from anaerobic effluents. Innovative, energy efficient nitrogen removal 18 processes are developing and involve several novel chemotrophic processes. This review 19 provides information on these processes, identifies how to control and retain the most desirable 20 microorganisms, and considers the impact of reactor configuration on performance. Given the 21 complexity of the technologies under development that remove nitrogen from anaerobically-22 treated domestic wastewater, we conclude that computational models can support their 23 development, and that sensor-mediated controls are essential to achieving energy efficiency.

24 **1. Introduction**

25 The domestic wastewater treatment industry in the United States is in a period of major change 26 motivated by two movements that are inspiring technological innovation in the industry: a need 27 to renew infrastructure that was built in response to the 1972 Clean Water Act and is reaching 28 the end of its design life, and a drive toward more sustainable practices including energy 29 recovery and efficiency. At the same time, increasingly stringent effluent nutrient guidelines are being implemented across the highly populated coastal margins of the United States¹⁻³, fueled 30 31 mostly by a need to mitigate eutrophication by managing nitrogen and phosphorus. Nutrient recovery technologies applied to domestic wastewater exist⁴ or are under development⁵⁻⁷. While 32

desirable from a sustainability perspective, nutrient recovery is typically not practical for
 treatment plants located far from agricultural land or where direct water reuse is not practiced
 because conveying liquid nutrients is economically infeasible⁸.

36 Although both nitrogen and phosphorus management are important, this review focuses on 37 mainstream biological nitrogen removal, which has been identified as a priority for innovation 38 and development by the water industry. This prioritization has been motivated by performance, 39 energy and space inefficiencies associated with existing nitrogen removing technologies that 40 evolved as "add ons" to conventional activated sludge processes. Within these traditional 41 systems, multiple microbial groups coexist across a range of redox environments to achieve 42 carbon and nitrogen conversions that are not optimized for any one group. Motivated by the 43 desire to develop cost-effective and energy-efficient technologies that achieve net energy neutral (or positive) domestic wastewater treatment, the water industry has prioritized the need to 44 45 develop a new generation of mainstream (as opposed to sidestream) nitrogen removing 46 technologies that are suitable for treating effluents from anaerobic systems receiving domestic 47 wastewater.

48 Anaerobic treatment of domestic wastewater is gaining attention for enhancing energy recovery from wastewater^{9, 10} because it: eliminates aeration, produces methane, and reduces sludge 49 handling requirements¹⁰. Several demonstrations have met USEPA's secondary effluent 50 standards¹¹⁻¹⁴ even at temperatures as low as $6^{\circ}C^{15}$; however, anaerobic processes produce 51 52 effluents with high ammonia-nitrogen, organic-nitrogen and dissolved methane concentrations which constrain wide-scale adoption of domestic anaerobic wastewater treatment¹⁶. Advancing 53 54 anaerobic treatment of domestic wastewater in nutrient-sensitive regions where total nitrogen 55 removal regulations exist or are anticipated requires implementing novel, energy efficient

56 mainstream nitrogen removal technologies that avoid nullifying energy savings realized from the 57 anaerobic process, while also mitigating greenhouse gas emissions, maintaining or reducing the 58 footprint, and reducing sludge handling requirements.

This review synthesizes past studies of biological nitrogen removal from anaerobic effluents¹⁷⁻²³ 59 60 and rapidly developing energy efficient approaches for nitrogen removal that have not been included in previous reviews^{24, 25}. We review microbial metabolic processes that are likely to be 61 62 active in nitrogen removing treatment systems receiving anaerobic effluents, and emphasize that 63 computational process modeling can help us understand how these complex metabolisms are 64 likely to behave under various treatment approaches. We consider tradeoffs that exist between 65 reducing energy demand, greenhouse gas emissions, space requirements and effluent nitrogen 66 concentration, all of which are strongly influenced by reactor configuration and operational 67 control strategy. Throughout the review, we highlight key areas where innovative research is 68 needed to develop solutions that meet increasingly stringent nitrogen regulations and move the 69 wastewater industry toward net energy neutral (or positive) treatment.

70 **2. Anaerobic effluent composition**

71 Direct anaerobic treatment of domestic wastewater generates an effluent (Table 1) with a 72 significantly different composition than that of conventional aeration-based treatment. For 73 example, ammonium and phosphate concentrations increase during anaerobic treatment due to ammonification and biotic²⁴ or abiotic^{24, 25} phosphate release. In contrast, conventional aerobic 74 75 treatment decreases ammonium and phosphate concentrations due to nitrification and uptake for 76 biomass growth. Sulfate concentrations in domestic wastewater vary considerably, depending 77 upon factors such as industrial wastewater discharges, coagulant used for drinking water 78 treatment, and drinking water sources. During anaerobic treatment, sulfate is reduced by sulfate

79 reducing microorganisms to sulfide and hydrogen sulfide gas, undesirable products that are 80 corrosive, malodorous with an odor threshold of 0.04 ppmv, dangerous to human health at concentrations above 100 $ppmv^{26}$, and inhibitory to nitrification²¹ and methanogenesis^{27, 28}. 81 82 Anaerobic treatment converts the organic carbon in domestic wastewater to gaseous products (carbon dioxide and methane) and dissolved products (organic $acids^{29, 30}$ and $methane^{11, 31}$). The 83 84 concentration of dissolved methane in anaerobic effluents varies seasonally and increases when 85 temperature decreases. While the average measured effluent concentration of dissolved methane is approximately 1.1 times saturation (Table 1), this value is highly variable and values as high as 86 4.1 times saturation^{15, 32} have been reported. Methane is a potent greenhouse gas and its release 87 represents a significant potential environmental impact¹⁶ that should be prevented. At present, 88 89 technologies to recover dissolved methane either produce biogas with low methane content or recover insufficient methane to offset their energy demands³³⁻³⁶. Considering the undesirable 90 91 impacts of sulfide and dissolved methane in anaerobic effluents, it makes sense to consider the 92 efficacy of using either or both as novel electron donors for nitrogen removal when stringent nitrogen discharge limits apply^{19, 37}. 93

94 Conventional nitrogen removal systems located in regions that require high levels of total 95 nitrogen removal tend to be electron donor limited due to use of primary clarifiers and aerobic loss of organic carbon as CO₂. Consequently, many full scale systems require exogenous 96 97 electron donor to achieve effluent nitrogen limits. In contrast, primary clarifiers are not used 98 when mainstream anaerobic treatment is deployed. Furthermore, the electron donor equivalents 99 typically present in anaerobic effluents are preserved in solution as organic COD not removed 100 during anaerobic treatment, dissolved methane and sulfide, and far exceed what is necessary to 101 achieve nitrogen removal via nitrite or nitrate (Table 1). This assumes these electron donors have

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102	not been aerobically oxidized and are available for denitrification, a challenge that can be
103	addressed through reactor configuration design (highlighted in Section 5). The COD balance also
104	indicates that the electron donors will compete for nitrite or nitrate, and underscores the
105	importance of applying reactor flow models, reaction rate kinetics and stoichiometry to ascertain
106	which competing metabolic groups will thrive during nitrogen removal from anaerobic effluents.

Table 1: Summary of effluent characteristics reported in studies of anaerobic domestic
 wastewater treatment, and electron donor requirements (expressed as COD equivalents) for
 complete denitrification from nitrate to N₂.

	Average ± Standard Deviation	Range	References ^{<i>a</i>}
Ammonium (mg N/L)	36±17	9-67	4, 17, 22, 23, 38-46
Phosphate (mg P/L)	6±7	1-20	4, 22, 38, 40, 41, 44, 45
Sulfide (mg COD/L)	62±83	3-184	4, 17, 34, 40, 41, 44, 47
Methane (mg COD/L)	91±50	42-204	31, 33-35, 40, 41, 46, 48
Soluble ^b COD (mg COD/L)	00 - 16	46.001	17, 23, 31, 34, 35, 38, 40-
	99±46	46-201	43, 45, 46
Total electron donor available ^c (mg COD/L)	252±107		
Electron donor consumed for denitrification via nitrate ^d (mg COD/L)	135±47		
Electron donor required for denitrification via nitrite ^d (mg COD/L)	98±32		

^a Only studies that treated real domestic wastewater were used to generate this table.

^bSoluble COD definition determined by the pore size of filters used: 0.45 μ m^{17, 35, 43, 45}, 0.7 μ m³⁸, Unreported^{23, 31, 34}, 40-42, 46

^cThe theoretical oxygen demand from ammonium is not considered in this calculation. Total electron donor is calculated as the sum of soluble COD, dissolved methane as COD, and sulfide as COD. Methane does not react in the COD test³⁴, and it was assumed that sulfide is oxidized or stripped in the filtration/sample acidification process prior to COD measurement.

^d The electron donor consumed incorporated demands for both respiration and cell growth, and considers loss of ammonium to support cell growth. Yields were assumed for nitrifiers (0.1 g COD_x/g N, Table 3), for heterotrophic denitrifiers applied only to the soluble COD fraction (0.39 g COD_x/g COD^{49}). Growth of other organisms was assumed to be insignificant.

110

111 **3.** Metabolic Pathways for Biological Nitrogen Removal from Anaerobic Effluents

- 112 A number of metabolic processes directly achieve or support nitrogen removal (Table 2) and
- 113 deserve consideration, although some produce environmentally undesirable byproducts like

nitrous oxide, a greenhouse gas 300 times more potent than carbon dioxide over 100 years⁵⁰ 114 115 (discussed further in Sections 3.2 and 4). Kinetic and stoichiometric parameters for many of the 116 reactions in Table 2 were compiled from the literature (see Table 3) and can be used with process 117 modeling to predict the distinct microbial community structures that are likely to form when 118 attempting nitrogen removal in anaerobic effluents. The majority of these parameters were 119 derived across a broad range of growth conditions; therefore, values vary considerably (see Table 120 S1), making predictions about competition between pathways challenging. Nevertheless, relative 121 values provide insight into competitive growth conditions that will occur in nitrogen removal 122 systems treating anaerobic effluents. When coupled with process flow models, the information 123 in Table 3 can help assess nitrogen removal treatment technologies under development, and 124 prioritize research or technology development needs for meeting nitrogen removal treatment 125 goals.

Table 2: Potential metabolisms present in a nitrogen removal system when sulfide, methane,
 acetate (a volatile fatty acid prominent in anaerobic effluents), and ammonium are present.

Relevant Processes		Metabolism	Catabolic Stoichiometric Equation	Free Energy of Reaction (1 bar, 25°C) ^a
Vitrification	Nitritation	Ammonium Oxidation ⁵³	$NH_4^++1.5O_2 \rightarrow NO_2^-+H_2O + 2H^+$	-190 kJ/mol NH4 ⁺
Z		Nitrite Oxidation ⁵³	$NO_2^- + 0.5O_2 \rightarrow NO_3^-$	-79 kJ/mol NO ₂
Denitrification	Denitritation	Anaerobic Ammonium Oxidation ^{b} (anammox) ⁵⁴	$\mathrm{NH_4}^+ + \mathrm{NO}_2^- \rightarrow \mathrm{N}_2 + 2\mathrm{H}_2\mathrm{O}$	-360 kJ/mol NH4+
		Denitrifying anaerobic methane oxidation (damo)	$3CH_4 + 8NO_2^- + 8H^+ \rightarrow 4N_2 + 3CO_2 + 10H_2O$	-1050 kJ/mol CH ₄
		Sulfide-based denitritation ⁵⁶	$3\text{HS}^{-}+5\text{H}^{+}+8\text{NO}_{2}^{-}\leftrightarrow 3\text{SO}_{4}^{-2}+4\text{N}_{2}+4\text{H}_{2}\text{O}$	-990 kJ/mol HS ⁻
		Heterotrophic denitritation ⁵³	$3CH_3COO^- + 8NO_2^- + 11H^+ \leftrightarrow 4N_2 + 6CO_2 + 10H_2O$	-1100 kJ/mol CH ₃ CHOO ⁻
		damo ⁵⁷	$5CH_4 + 8NO_3^- + 8H^+ \rightarrow 4N_2 + 5CO_2 + 14H_2O_3^-$	-830 kJ/mol CH ₄
	Denitratation	Sulfide-based denitrification ⁵⁶	$5\text{HS}^{-}+3\text{H}^{+}+8\text{NO}_{3}^{-}\leftrightarrow 5\text{SO}_{4}^{-2}+4\text{ N}_{2}+4\text{H}_{2}\text{O}$	-770 kJ/mol HS ⁻
		Heterotrophic denitrification ⁵³	$5CH_3COO^- + 8NO_3^- + 13H^+ \leftrightarrow 4N_2 + 10CO_2 + 14H_2O$	-910 kJ/mol CH ₃ CHOO ⁻
ic	ses	Methane oxidation ⁵⁸	$CH_4 + 2O_2 \rightarrow CO_2 + 2H_2O$	-820 kJ/mol CH ₄
rob	ces	Sulfide oxidation ⁵⁹	$HS^{-}+2 O_2 \leftrightarrow SO_4^{-2}+H^+$	-760 kJ/mol HS
Ae	pro	Heterotrophic oxidation ⁶⁰	$\mathrm{CH}_3\mathrm{COO}^-\!\!+2\mathrm{O}_2\!\!+\mathrm{H}^+ \leftrightarrow 2\mathrm{CO}_2\!\!+\!2\mathrm{H}_2\mathrm{O}$	-890 kJ/mol CH ₃ CHOO ⁻
		Sulfate reduction ⁶¹	$CH_3COO^- + SO_4^{2-} + 2H^+ \leftrightarrow HS^- + 2CO_2 + 2H_2O$	-140 kJ/mol CH ₃ CHOO

128

^a The free energy of reaction, which was calculated using⁵¹, can be used to determine the true growth yield for each process⁵².

^bNitrate is produced from anammox as a result of anabolic reactions.

129	Table 3: Median kinetic (maximum specific growth rate, μ_{max} , half-saturation constants for electron
130	donor, K _{donor} , and acceptor, K _{acceptor}) and stoichiometric (cell growth yield) parameters for relevant
131	biological processes. Underlined values were obtained with pure cultures. With the exception of + which
132	have a temperature range between 20-38° C, all kinetic and stoichiometric parameters were determined
133	between 17-30° C. For all references and ranges see supplementary information.

Organism	$\begin{array}{c} \mu_{max} \\ (day^{-1}) \end{array}$	Observed Yield		K _{donor}			Kacceptor				
	Median	Median	Units	Median	Donor	Units	Median	Acceptor	Units		
Ammonium Oxidizing											
Microorganisms	1.16^{62}	0.12^{63}	g COD _x /g N	0.7^{64}	$\mathrm{NH_4}^+$	mg N/L	0.8^{49}	O_2	mg O ₂ /L		
(AOM)											
Nitrite Oxidizing	0.65	0.0849	a COD /a N	0.0	NO -	ma N/I	1 60	0	mg O /I		
Bacteria (NOB)	0.65	0.08	g COD _x /g N	0.9	NO_2	Ing N/L	1	O_2	ling O₂/L		
Methane Oxidizing	3 065	0.06 ⁶⁶	a COD /a COD	0.4467	СЦ	mg COD/I	0.05	0	mg O /I		
Bacteria	<u>3.0</u>	0.00	$g COD_{x}/g COD_{s}$	<u>0.44</u>	CH_4	ling COD/L	0.05	O_2	ling O₂/L		
Sulfide Oxidizing	3.25	2 25	2 25	0.2261	a COD /a COD	21	<u>цс-</u>	ma S/I	259, 68	0	ma O /I
Bacteria		0.52	$g COD_{x}/g COD_{s}$	21	пэ	mg 5/L	5	O_2	$\log O_2/L$		
Nitrite-damo	0.035	0.038	$g COD_x/g COD_s$	0.32^{69}	CH ₄	mg COD/L	0.6^{70}	NO ₂ ⁻	mg N/L		
Nitrate-damo+	0.8	0.09	$g COD_x/g COD_s$	5.44 ⁷¹	CH_4	mg COD/L	0.11^{72}	NO ₃ ⁻	mg N/L		
Anammox ⁺	0.121	0.11	g COD _x /g N	0.173	NH_4^+	mg N/L	0.27	NO ₂	mg N/L		
Sulfide-based denitrification	3.24 ⁷⁴	1.4	g COD _x /g S	1.1	HS⁻	mg S/L	5.5	NO ₃ -	mg N/L		

134

135 **3.1** Competition for oxygen in downstream nitrogen removal systems

136 The reduced nitrogen in anaerobic effluents has to be oxidized to produce nitrite or nitrate to achieve anaerobic biological nitrogen removal. Directing nitrification processes through nitrite⁷⁵⁻ 137 ⁷⁷ (nitritation) as opposed to nitrate (nitratation) is preferable because it reduces aeration demand 138 139 by 25% based on stoichiometric estimates. If partial nitritation is desired, where about half of the 140 ammonium is oxidized to nitrite so that both are present to support anammox, then stringent 141 control strategies or novel reactor configurations are needed, as discussed in section 4 and 5. 142 Once oxygen is introduced, however, other competing microbially-mediated aerobic reactions 143 also occur. For instance, dissolved methane coupled with aeration supports the growth of 144 methanotrophic bacteria. Information in Table 3 suggests that aerobic methanotrophs (median K₀₂=0.05 mg/L) will out-compete ammonia oxidizing microorganisms (AOM) (median K₀₂=0.8 145 mg/L) for oxygen, which has been experimentally corroborated $^{78, 79}$. Although aerobic 146

methanotrophy represents an aeration (and, therefore, energy) demand that does not contribute to
nitrogen removal, it also mitigates greenhouse gas emissions by preventing methane stripping.
Sulfide exerts an abiotic oxygen demand because sulfide is rapidly oxidized in the presence of
trace metals^{80, 81} but sulfide oxidizing bacteria are not expected to compete for oxygen with
nitrifiers and methane oxidizing bacteria, based on the values in Table 3. Finally, any residual
VFA will be rapidly oxidized by heterotrophic bacteria under aerobic conditions at a faster rate
than all other biological processes listed in Table 3⁴⁹.

154 Multiple factors (in addition to aeration) can be used to control the growth competition between 155 AOM and NOB (see Section 4). Furthermore, bioreactor design (discussed in Section 5) can 156 influence which metabolic processes succeed. Overall, it is clear that there are complex 157 interactions between a number of aerobic microbial metabolic processes that occur when treating 158 methane- and sulfide-ladened anaerobic effluents and will contribute to or interfere with the goal 159 of nitrogen removal. This strengthens the argument for using computational models of new 160 treatment technology concepts to elucidate likely competitive responses, and the need for good 161 kinetic and stoichiometric estimates to do so.

162 **3.2 Potential denitrification pathways**

163 Biological nitrogen removal processes use an electron donor to reduce nitrite or nitrate,

164 preferably to innocuous N₂ gas. Conventional wastewater treatment systems use organic matter

165 as the electron donor for heterotrophic denitrification to achieve nitrogen removal. Excellent

166 nitrogen removal (i.e., total nitrogen concentrations below 4 mg N/L) can be achieved when the

- 167 influent COD/NH₃ ratio is > twelve⁴⁹. In many instances where conventional treatment
- approaches are used, wastewater streams are organic carbon-limited and supplemental organic
- 169 matter must be provided at an economic and environmental life cycle $cost^{82, 83}$. Conversely,

170 nitrogen removal from anaerobic effluents can proceed via multiple electron donors that include 171 dissolved methane, ammonium, and sulfide in addition to residual biodegradable organic matter that exists mostly as volatile fatty acids (VFAs)^{29, 30}. As noted in Table 1, anaerobic effluents 172 173 have very high COD/NH₃ ratios and, therefore, typically have sufficient electron donor to meet 174 excellent total nitrogen removal goals. Note, however, that methane and sulfide represent well 175 more than 50% of the electron donor in these effluents, although full-scale systems have not yet 176 been designed that use them to achieve mainstream nitrogen removal. Herein lies an opportunity 177 for innovative nitrogen removal via one or more of these novel electron donors.

178 Anammox (ammonium as electron donor) and damo (methane as electron donor) are oxygensensitive^{84, 85} anaerobic microbial metabolic processes that can achieve nitrogen removal. While 179 180 our understanding of how to use anammox for denitritation in mainstream applications is growing^{76, 77, 86-89}, damo-based mainstream nitrogen removal is in its infancy^{19, 20}. Nitrite or 181 182 nitrate can be used as electron acceptor to oxidize methane (damo) while meeting nitrogen 183 removal goals and reducing methane emissions. Nitrous oxide emissions from damo have not been reported⁹⁰ and, in the case of nitrite-damo, are unlikely to be produced because the 184 enzymatic denitritation pathway does not involve nitrous oxide⁹¹. Similarly, nitrous oxide 185 emissions are diminished in anammox systems^{92, 93}. Nitrite-based damo has a particularly slow 186 187 maximum specific growth rate relative to nitrate-based damo and anammox (Table 3), and both 188 damo and anammox have slow maximum specific growth rates relative to heterotrophic 189 denitrification, suggesting that long solids residence times (SRTs) are needed to retain the 190 former. Between the demand for long SRTs and the dilute biomass concentrations expected in 191 mainstream damo or anammox applications versus side stream (high strength) applications,

biofilm treatment process configurations should be used⁹⁴. Indeed, attempts to limit biomass washout of nitrite-damo with biofilms showed improved nitrite reduction rates²⁰.

194 Both experiments and modeling exercises offer insight into how damo and anammox may 195 compete in systems treating anaerobic effluents. Early enrichments of suspended growth co-196 cultures of anammox and nitrite-dependent damo suggested they would only coexist under ammonium-limited conditions⁹⁵, which slow anammox growth. Modeling results support this 197 198 finding and show that anammox out-competes damo for nitrite at higher ammonium concentrations⁷⁰. Other suspended and biofilm enrichment studies found that nitrate- and to a 199 lesser extent nitrite-dependent damo and anammox coexist^{96, 97}. Nitrate-dependent damo and 200 201 anammox can coexist because they do not compete for substrates; furthermore, damo can use the nitrate produced by anammox 96 . In an interesting twist, aerobic methanotrophs can support 202 203 heterotrophic denitrification by converting dissolved methane into methanol, which can be used as an electron donor (reviewed by^{98}). One study found an aerobic methanotroph that reduced 204 205 nitrate to nitrous oxide under low oxygen conditions⁹⁹. Others showed that aerobic methanotrophs can oxidize ammonium to nitrite¹⁰⁰ and emit nitrous oxide in the process^{101, 102}; 206 207 however, it is unclear if this process can successfully compete with AOMs to achieve partial 208 nitritation of anaerobic effluents.

While the competition between anammox and damo is complex and multifaceted, addition of
sulfide oxidation makes it more so. Microbial sulfide oxidation via nitrite and nitrate is very
rapid relative to other processes that use these same electron acceptors (reviewed by ¹⁰³). For
example, denitrification rates are faster with sulfide as the electron donor than with methanol³⁷.
Additionally, sulfide can be partially oxidized to elemental sulfur or sulfite, and both can be used
as an electron donor for denitrification^{104, 105}. In one study of industrial wastewater, 90% total

215 nitrogen removal when anammox and sulfide-based denitrification via nitrate (produced by the anammox process) were coupled¹⁰⁶. Interestingly, sulfide inhibits AOM and NOB at 216 concentrations around 3 and 1 mg S/L^{107} , respectively; therefore, differential sulfide-driven 217 inhibition of NOB over AOM could be a strategy for achieving nitritation¹⁰⁸. Sulfide also inhibits 218 219 anammox, but studies are conflicted over how sensitive it is since inhibition half saturation constants (K_I) range from 0.3^{109} to 30^{110} mg S/L. Finally, sulfide inhibits heterotrophic reduction 220 of nitrous oxide to nitrogen gas and can lead to increased nitrous oxide emissions¹¹¹⁻¹¹³. In total, 221 222 sulfide plays a very complex yet important role during nitrogen removal from anaerobic effluents 223 and also offers interesting possibilities as a useful electron donor; in turn, it is a high priority for 224 further study.

225 4. Aeration Control to Sustain Nitritation

226 Removing nitrogen from anaerobic effluents using nitrite as the electron acceptor can save 227 energy by reducing aeration, but requires a strategy that out-selects (or prevents growth of) NOB. 228 Out-selection strategies have been demonstrated for side-stream treatment of anaerobic digester reject waters and include using elevated temperatures^{114, 115}, high free ammonia (FA) 229 concentrations¹¹⁶⁻¹¹⁸, high free nitrous acid concentrations¹¹⁷⁻¹¹⁹, and high pH¹²⁰. However, these 230 231 strategies are unsuitable for mainstream processes, which have ammonium concentrations up to 232 50 times lower than anaerobic digesters that treat waste sludge, have large volumes that are 233 impractical to heat, and are too dilute for FA inhibition. Therefore, alternative strategies tailored 234 to mainstream applications are needed.

235 Given the higher relative median oxygen affinity (lower K₀₂) of AOM over NOB (Table 3),

dissolved oxygen (DO) control has been used to achieve stable nitritation in sidestream¹²¹ and

mainstream¹²² processes. The actual DO used to achieve NOB out-selection is complicated by 237 the fact that oxygen affinity varies among different genera of NOB^{76, 123, 124}. Several mainstream 238 studies have successfully out-selected NOB at DO concentrations at or greater than 1.5 mg/ L^{23} , 239 ^{76, 86, 88, 125}. It has also been shown that NOB exhibit a period of reduced growth rate following 240 anoxic disturbances^{125, 126} (called transient anoxia), and this phenomenon has been used to 241 achieve NOB out-selection⁷⁶. Using transient anoxia to support nitrite-mediated nitrogen 242 243 removal from anaerobic effluents requires consideration of the dissolved VFAs, methane and 244 sulfide present in anaerobic effluents, which can aid in achieving rapid anoxia when they are 245 biotically or abiotically oxidized via DO. A key development that has enhanced the application of nitritation in mainstream processes is 246 247 sensor-mediated online control systems that are reliable and durable. Newer, more robust sensor technologies for monitoring ammonia^{76, 127}, DO^{76, 127}, nitrite¹²⁸ and nitrate^{76, 127, 128} are being used 248 successfully in full-scale systems and increasingly for nitritation control^{76, 129, 130}. Although 249 250 online control for mainstream nitrogen removal from anaerobic effluents is in its infancy with no 251 published studies to date, it has been shown that sensor-mediated control can help achieve stable, mainstream anammox downstream of aerobic carbon removal⁷⁶; these experiences can inform 252 253 strategies for sensor-mediated online control of nitrogen removal from anaerobic effluents. An

example of sensor- mediated online control is ammonia-based aeration control where on-line ammonia sensors are used to control the duration of aeration⁸⁶ or DO setpoint⁸⁵ to achieve NOB out-selection and ensure partial nitritation where only a fraction of the ammonium is oxidized to nitrite so that both nitrite (electron acceptor) and ammonium (electron donor) coexist to support anammox. In one pilot scale study⁷⁶, the duration of aeration was decreased when the ammonium to nitrite plus nitrate ratio was less than one. Subsequently, aeration was gradually

260 increased again once a predetermined minimum duration was reached. In essence, this strategy 261 controlled the aerobic solids residence time (SRT) to only allow ammonium oxidation, and was 262 combined with transient anoxia to improve NOB out-selection. The innovative combination of 263 ammonia-based control of aerobic SRT and transient anoxia should also help achieve NOB out-264 selection in anaerobic effluents. Another control strategy for nitrite-mediated nitrogen removal in mainstream applications involves pH-based control¹³¹. For example, pH-based control of 265 266 feeding and aeration was applied to a lab-scale sequencing batch reactor system modeled after the DEMON[®] process¹³² but adapted for mainstream applications⁷⁷. It was deemed successful 267 268 because higher total nitrogen removal was achieved and less air was used than a system with 269 time-based aeration control. Both of these studies of sensor-mediated control were operated at 270 ambient temperatures (20°C and 25°C); NOB out-selection at lower temperatures is an area of on-going research^{133, 134}. 271

272 While transient anoxia may be used to out-select NOB, some studies have found it promotes nitrous oxide emissions¹³⁵ (reviewed by ¹³⁶). If transient anoxia is to be implemented, frequent 273 aerobic-anoxic cycling may be needed because it decreases nitrous oxide emissions¹³⁷. There is 274 275 limited understanding of how process control can be used to minimize nitrous oxide emissions, 276 which is confounded by the dynamic nature of nitrous oxide emissions from wastewater treatment plants¹³⁸. In general, high DO concentrations and one-step systems (one reactor is used 277 278 with multiple redox zones to achieve all biological treatment goals) instead of multi-step 279 systems (wastewater passes between reactors and the biomass in each is retained and not mixed) reduce nitrous oxide^{139, 140}. These findings suggest that nitrous oxide emissions and energy 280 281 consumption to achieve nitrogen removal from anaerobic effluents are inter-related and should 282 be evaluated simultaneously when reviewing system control strategies.

5. Reactor Configurations

284 Achieving mainstream nitrogen removal from anaerobic effluents offers unique challenges that 285 demand innovative solutions. First, because anaerobic effluents contain significant amounts of 286 dissolved gases that are difficult to recover for beneficial purposes and damaging if stripped, 287 successful configurations should minimize off-gassing. Second, innovative, energy efficient, 288 mainstream nitrogen removal can occur by either one-step (nitritation plus denitritation together) 289 or two-step (nitritation and denitritation occur separately) processes. These reactor 290 configurations utilize multiple complex microbial metabolisms that achieve nitrogen removal, 291 support NOB out-selection, reduce energy consumption, prevent greenhouse (including nitrous 292 oxide) and corrosive gas emissions, and reduce space requirements. For treatment plants that use 293 aerobic treatment to manage organic carbon, the most energy efficient means to achieve denitrification is by anammox¹⁴¹; both one- and two-step technologies are being developed 294 around this strategy^{76, 86, 87, 142}. However, for systems that treat organic carbon anaerobically, 295 296 multiple electron donors capable of denitrification are present and dissolved gases may replace 297 or supplement ammonia as an electron donor.

298 **5.1 Designing reactor configurations to minimize gaseous stripping**

The dissolved gases in anaerobic effluents are prone to off-gassing. Bubbling air into a reactor where influent concentrations of dissolved gasses are highest makes a system more vulnerable to gas stripping¹⁴³, and prevents use of the dissolved gases as electron donors. Reactor configurations that avoid use of bubbles or extensive agitation during anaerobic nitrogen removal are needed and some promising options are discussed in Sections 5.2 and 5.3. Furthermore, advances in technologies that recover dissolved methane from anaerobic effluents in an energy efficient manner without diluting the gas with carbon dioxide and hydrogen sulfide are greatly needed but, to date, do not exist³³. If a less energy intensive method for recovering dissolved methane is developed, enough electron donor is likely to remain in anaerobic effluents to support nitrogen removal from domestic wastewater via sulfide and VFA ($161 \pm 95 \text{ mg/L}$ COD electron donor available, well above nitrite or nitrate-based denitrification demand as given in Table 1); however, gas stripping should still be avoided to prevent loss of hydrogen sulfide. Consequently, reactor configurations that prevent gas stripping and encourage biological

312 reactions that consume dissolved gases are needed.

313 **5.2 One-step nitrogen removal systems**

314 We contend that biofilms are preferred for one-step systems designed to achieve anaerobic 315 nitrogen removal because slow growing and low yielding anammox, damo and anaerobic sulfur 316 oxidizing microorganisms would be retained more effectively than in suspended culture systems. 317 Because nitrite is the preferred electron acceptor for energy efficient nitrogen removal, one-step 318 systems must be configured so that off-gassing of dissolved electron donors is prevented even 319 though aeration is needed to achieve nitritation. Furthermore, AOM must coexist with anaerobic 320 nitrogen removing microorganisms in one-step systems; unfortunately, biofilm systems make NOB out-selection difficult, but not impossible^{86, 87, 92}, to achieve. Some biofilm systems are co-321 322 diffusional, where the electron donor and electron acceptor diffuse into the biofilm in the same 323 direction. In these systems, aeration to support biological nitritation occurs in the bulk liquid and 324 creates an aerobic outer biofilm layer and anoxic inner layer. To prevent off-gassing of precious 325 electron donors from the bulk liquid, an anoxic phase is needed at the beginning of the process 326 where feed is introduced. We suspect that sequencing between anoxic and aerobic conditions 327 will prevent damo from establishing in co-diffusional one-step systems in lieu of other 328 denitrifiers and that methane oxidation and stripping may still occur during the nitritation phase.

329 Two types of co-diffusional, one-step biofilm technologies that show great promise to achieve 330 energy efficient nitrogen removal from anaerobic effluents are highlighted: granular sludge systems¹⁴⁴ select for dense biofilm aggregates in a sequencing batch reactor that is exposed to 331 332 high shear forces and managed with short settling and decant times, and moving bed biofilm reactors (MBBRs)¹⁴⁵ contain an inert carrier that supports biofilm formation. Both technologies 333 are compact have been successfully applied at full-scale for nitrogen removal from centrate¹⁴⁶, 334 ¹⁴⁷, and are being evaluated for use in mainstream systems that couple aerobic management of 335 carbon, nitritation and anammox $^{86, 142, 148}$; therefore their use for nitrogen removal from 336 337 anaerobic effluents would be a new application of the technology. 338 In contrast to granular sludge systems and MBBRs, membrane biofilm reactors (MBfRs) are 339 one-step biofilm systems that are ideally suited to treat anaerobic effluents. In these systems, 340 oxygen diffuses through biofilm-coated hollow fiber membranes and creates a bubbleless aerobic zone adjacent to the membrane and an anaerobic zone at the biofilm surface. If used with an 341 342 anaerobic effluent, the electron donors would diffuse from the anaerobic bulk liquid into the 343 biofilm across anaerobic then aerobic zones. In contrast, electron acceptors diffuse from the 344 membrane lumen or are biological formed and then diffuse into the biofilm in the opposite direction; hence, these systems are counter-diffusional¹⁴⁹. The biofilm-coated membrane 345 provides bubbleless aeration, which prevents gas stripping^{150, 151}, and the counter-diffusion 346 347 configuration allows for separate control of electron donor mass loading rates and aeration rates 348 via the membrane lumen. These systems achieve efficient aeration; one study showed oxygen transfer efficiencies of 20-35%¹⁵². With one exception¹⁵³, nitrogen removal from wastewater 349 using MBfRs has been limited to lab-scale demonstrations of high strength wastewater^{92, 154, 155} 350

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and therefore the technology is still under development; however; it holds great promise as long
as reliable NOB out-selection^{149, 156, 157} can be achieved.

353 **5.3 Two-step nitrogen removal systems**

354 Separating nitritation and denitritation steps offers simpler control, allows for a combination of 355 suspended and/or biofilm processes to be used, and may be the easiest to deploy by retrofitting 356 existing infrastructure; however, it requires a larger treatment footprint. To prevent loss of 357 dissolved electron donor gases, a denitritation tank can precede the nitritation tank and achieve 358 nitrogen removal via recycled or internally recirculated nitrite using a configuration that 359 resembles a Modified Ludzack Ettinger (MLE) system. Although internal recirculation provides more nitrite flux and higher levels of total nitrogen removal⁴⁹ it comes at an energy cost; one 360 361 study of an MLE process requiring 10 mg/L effluent total nitrogen found that the energy required 362 for recirculation pumping comprised 9% of the total energy needed to operate the process¹⁵⁸. For 363 the denitritation step, a biofilm system will retain the slow-growing denitrifying microorganisms 364 best along with VFA-consuming denitrifiers. A number of energy efficient configurations could be used, including MBBRs or fluidized bed systems, where an inert carrier supports biofilm 365 366 formation.

367 6. Moving Forward

368 Coupling nitrogen removal with anaerobic treatment of domestic wastewater requires improved 369 understanding of the key microbial metabolic processes involved and presents opportunities to 370 create novel reactor configurations and sensor-based control strategies. We contend that 371 bioprocess models, which can be validated in lab- and pilot-scale studies, are an integral step to 372 developing these treatment technologies because they help us understand how different reactor

373 configurations and operating strategies control the complex microbial population interactions 374 within them. Significant research efforts are underway to identify functional and reliable reactor 375 configuration and operational strategy combinations that support the microbial metabolisms 376 needed to achieve energy efficient nitrogen removal goals. Advancing these technologies is 377 critical to making energy neutral or positive wastewater treatment via anaerobic mainstream 378 treatment economically viable in many populated regions around the world where total nitrogen 379 regulations are in place or anticipated for wastewater treatment systems.

380 Acknowledgements

381 We gratefully acknowledge our funding source the Water Environment Research Foundation

382 (ENER4R12). J.D.V. and L.B.S. were both supported by graduate research fellowships from the

383 U.S. National Science Foundation and the University of Michigan Horace H. Rackham School of

384 Graduate Studies.

385 Supporting Information Available

- 386 The Supporting Information includes complete kinetic and stoichiometric parameter literature
- 387 review and schematics of the reactor configurations considered. This material is available free of
- 388 charge via the Internet at http://pubs.acs.org/journal/estlcu.
- 389 The authors declare no competing financial interest.

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839 Prospects for Biological Nitrogen Removal from Anaerobic Effluents during Mainstream

840 Wastewater Treatment

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